



INNOVATIVE SOLUTION FOR MINI-MICRO-TRENCHING SYSTEMS

A. Corradini, G. Cerni, F. Montalti, D. Bazzucchi, A. Bruno, S. Barbetti, G. Franceschetti, D. Barbetti

25th May 2023











DEPARTMENT OF CIVIL AND ENVIRONMENTAL ENGINEERING

- Alessandro Corradini
- Gianluca Cerni





Expertise in innovation

- Francesco Montalti
- Daniel Bazzucchi
- Antonio Bruno





- Stefano Barbetti
- Gabriele Franceschetti
- Davide Barbetti











- Fiber to the Home
- No Dig technologies

EXPERIMENTAL PROGRAM

- Objectives
- Material
- Testing methods
- Sample preparation

EXPERIMENTAL RESULTS

- Preliminary tests
- Micro and Macrotexture tests
- ITS tests

CONCLUSIONS







- Telecommunication networksNervous system of any modern nation
- High-speed internet
- Infrastructure
 Services

Raw materials of the future, for the entire economic and social environment









FTTH (Fiber To The Home)

- High speed for add-on services (Video on demand, Online Gaming, HDTV)
- Future-proof solution.

Over the last decade, the technological development concerning such networks has led to optical fiber cables with greater efficiency and smaller sizes.

Miniaturization of primary elements

Improved digging technologies









No-dig technologies are a set of techniques ideal for reline, repair and renew the underground utilities network minimizing the impact on road pavements.

Unlike the traditional trench, those techniques (MINI and MICRO-TRENCHES):

- ➢ faster execution → lower production of waste material;
- ➤ reduced excavation section → minor damage on existing pavement;
- Iower recovery times and costs.









The quality of backfilling plays an important role in the performance of the trench.

Material used:

- Granular mixtures
- Cement grout
- Cement concrete
- Bituminous mixtures

Backfilling <u>Not suitable</u> materials <u>Not properly installed</u>

PREMATURE PAVEMENT FAILURE











• EXPERIMENTAL PROGRAM - Objective

OBJECTIVE \rightarrow Investigation on the suitability of a new technology for mini-microtrenching applications which based on the use of a **CEMENTITIOUS BITOMINOUS GROUT** as backfilling material.

Preliminary tests:

Characterization of the grout (fresh and hardened).

Application on road pavements as backfilling material:

- Evaluation of MICRO and MACROTEXTURE on laboratory manufactured slabs;
- ➢ Investigation on mini and micro-trenching systems in terms of ADHESION AT THE INTERFACE → crucial aspect to ensure longer lasting systems.





EXPERIMENTAL PROGRAM - Material

Cementitious-bituminous grout \rightarrow Flowmix Fast, patented by CVR S.p.A.

Main components:

Highly reactive hydraulic binders, bituminous binder, fine aggregate, micro silicates with pozzolanic behaviour, expansive agents, specific no segregating and superplasticizers additives.

- Siliceous and carbonate aggregates with a controlled 0/4 mm gradation;
- The main hydraulic binder is an ordinary Portland cement CEM IIA LL 42.5R;
- Cationic slow setting bituminous emulsion:

| Test | Value | Reference Standard |
|----------------------|---------|--------------------|
| Bitumen content (%) | 60 | UNI EN 1428 |
| Penetration (dmm) | <100 | UNI EN 1426 |
| Softening point (°C) | 43 | UNI EN 1427 |
| Adhesiveness (%) | >80 | UNI EN 13614 |
| Density (Kg/m³) | 1000±50 | - |





• EXPERIMENTAL PROGRAM - Material

Cementitious-bituminous grout \rightarrow Flowmix Fast \rightarrow One-time backfillig









EXPERIMENTAL PROGRAM – Methods

PRELIMINARY TESTS

| Test | Reference Standard | | | | | | |
|--|---------------------------------|--|--|--|--|--|--|
| FRESH GROUT | | | | | | | |
| Air | UNI EN 1015-7 | | | | | | |
| Spreading | UNI EN 1015-3 | | | | | | |
| Setting start time | UNI EN 196-3 | | | | | | |
| End of grip time | UNI EN 196-3 | | | | | | |
| Fresh grout density | UNI EN 1015-6 | | | | | | |
| HARDENED GROUT | | | | | | | |
| Hardened grout density | UNI EN 1015-10 | | | | | | |
| Average flexural strength at 2 hours | | | | | | | |
| Average compressive strength at 2 hours | UNI EN 1015-11 | | | | | | |
| Average flexural strength at 28 days | | | | | | | |
| Average compressive strength at 28 days | | | | | | | |
| Average resistance to adhesion after 28 days on concrete "f _u " | UNI EN 1015-12 | | | | | | |
| Characteristic value of the initial shear strength "f _{v0k} " | Tab.11.10.VIII NTC DM 17/1/2018 | | | | | | |
| Water absorption by capillarity coefficient "C _m " | UNI EN 1015-18 | | | | | | |
| Modulus of elasticity in compression "E" | UNI EN 13412 | | | | | | |
| Shrinkage at 28 days | UNI 8147 – B method | | | | | | |







• EXPERIMENTAL PROGRAM – Methods

MICROTEXTURE and MACROTEXTURE

Aggregate physical properties affect road pavement surface charactheristics which are in turn related to pavement functionality. Such charactheristics are related to the geometrical profile of the pavement on the vertical level.

- The micro-texture has a wavelength up to approximately 0.5 mm;
- The macro-texture, from 0.5 to 50 mm;



microtexture l < 0.5 mm









• EXPERIMENTAL PROGRAM – Methods

MICROTEXTURE \rightarrow UNI EN 13036-4

An apparatus for stationary measurements of slip/skid resistance of a surface is the SKID TESTER (BRITISH PENDULUM)

The device consists of a pendulum arm that has a spring-loaded standard rubber slider to its end. On release of the pendulum from a horizontal position, the loss of energy as the slider assembly passes over the test surface is measured by the <u>reduction in</u> <u>length of the upswing</u> using a calibrated scale \rightarrow **PTV value**

The arm is released from its horizontal position to swing over a contact path length of 126 ± 1 mm.







EXPERIMENTAL PROGRAM – Methods

MACROTEXTURE → UNI EN 13036-1

Measurement of texture depth by volumetric technique (sand patch method).

A known volume (>25 ml) of graded glass spheres is poured on the dry and clean pavement surface. These are spread on the surface through a spreading tool, filling the surface voids. A circular patch is determined so as the spreading tool touches the surface particle tips.

The mean texture depth (mm) MTD is

 \rightarrow M.

 $\mathsf{MTD} = 4 \times V / (\pi \times D^2)$

- V is the volume of spheres (mm3),
- *D* is the average diameter (at least 4 values) of the area covered by the material (mm).









• EXPERIMENTAL PROGRAM – Methods

INDIRECT TENSILE STRENGTH (ITS) TEST \rightarrow UNI EN 12697-23

A cylindrical specimen is brought to the <u>specified test temperature</u> placed in the compression testing machine between the loading strips, and loaded diametrically along the direction of the cylinder axis with a <u>constant speed of displacement (50 mm/min)</u> until it breaks.

Temperature = 25 °C

VERTICAL DIAMETER ≅ INTERFACE









EXPERIMENTAL PROGRAM – Methods

INDIRECT TENSILE STRENGTH (ITS) TEST \rightarrow UNI EN 12697-23

The indirect tensile strength is the maximum tensile stress calculated based on:

- ➤ the peak load applied at break Pmax
- \succ the dimensions of the specimen (h, D).

$$ITS = \frac{2Pmax}{\pi hD}$$









• EXPERIMENTAL PROGRAM – Sample preparation

MICROTEXTURE and MACROTEXTURE tests

- > Slab: length x width x heigth \rightarrow 36cmx36cmx1cm
- > 2 replicates









- EXPERIMENTAL PROGRAM Sample preparation
- INDIRECT TENSILE STRENGTH test
- ightarrow Adhesion at the interface
- 3 types of samples: CMT, MMT, CP

Trial site – Gubbio (Pg, Italy)

➤ CMT → Cored from mini or microtrenches











 EXPERIMENTAL PROGRAM – Sample preparation

INDIRECT TENSILE STRENGTH test

ightarrow Adhesion at the interface

3 types of samples: CMT, MMT, CP

Trial site – Gubbio (Pg, Italy)

➤ CMT → Cored from mini or microtrenches













 EXPERIMENTAL PROGRAM – Sample preparation

INDIRECT TENSILE STRENGTH test

ightarrow Adhesion at the interface

3 types of samples: CMT, MMT, CP

Trial site – Gubbio (Pg, Italy)

➤ CMT → Cored from mini or microtrenches

CPu



CPb











 EXPERIMENTAL PROGRAM – Sample preparation

INDIRECT TENSILE STRENGTH test

ightarrow Adhesion at the interface

3 types of samples: CMT, MMT, CP

Trial site – Gubbio (Pg, Italy)

> MMT

- Coring on mini or microtrenches <u>prior</u> to backfilling → Half specimen
- Trimming → MMTw MMTb
- Setting up specimen into a plastic tube over a plane surface
- Casting
- Curing (14 days)







| Test | Value | | | | | | | |
|--|--|--|--|--|--|--|--|--|
| FRESH GROUT | | | | | | | | |
| Air | ≤10% | | | | | | | |
| Spreading | 165 mm | | | | | | | |
| Setting start time | 15 minutes | | | | | | | |
| End of grip time | 25 minutes | | | | | | | |
| Fresh grout density | 2050 kg/m ³ | | | | | | | |
| HARDENED GROUT | | | | | | | | |
| Hardened grout density | 1900 kg/m ³ | | | | | | | |
| Average flexural strength at 2 hours | ≥ 1,0 N/mm² | | | | | | | |
| Average compressive strength at 2 hours | ≥ 4,0 N/mm² | | | | | | | |
| Average flexural strength at 28 days | ≥ 3,5 N/mm² | | | | | | | |
| Average compressive strength at 28 days | ≥ 18,0 N/mm² | | | | | | | |
| Average resistance to adhesion after 28 days on concrete "f _u " | ≥ 0,80 N/mm² - A | | | | | | | |
| Characteristic value of the initial shear strength "f _{vok} " | ≥ 0,30 N/mm ² | | | | | | | |
| Water absorption by capillarity coefficient "C _m " | ≤ 0,20 kg/(m ² min ^{0,5}) | | | | | | | |
| Modulus of elasticity in compression "E" | ≥ 16 GPa | | | | | | | |
| Shrinkage at 28 days | ≤ 300 μm | | | | | | | |





- \succ Fluid consistency \rightarrow monolitic backfilling;
- \succ Lack of bleeding phenomena \rightarrow homogeneity;
- Walkability and trafficability in 40 minutes and 50 minutes, respectively;
- Expansive behaviour to ensure compensated shrink;
- Lack of lowering phenomena.









• EXPERIMENTAL RESULTS - Microtexture test

- 2 slabs were tested
- 4 testing positions (from 1 to 4)
 - 5 cm far from each edge
 - half of its width
- 2 sliding directions
 - clockwise (A)
 - counter-clockwise (B)
- 5 measuremnts were carried out for each test position











• EXPERIMENTAL RESULTS - Microtexture test

| SLAB "1" | | | | | | | | | | | | | | |
|--|-----------|-------|-------|----|----|----|---------|-----------|----|----|----|----|----|-----|
| | Sliding | Swing | | | | | Sliding | Swing | | | | | | |
| Edge | direction | 1 | 2 | 3 | 4 | 5 | PTV | direction | 1 | 2 | 3 | 4 | 5 | PTV |
| 1 | А | 69 | 67 | 68 | 67 | 68 | 68 | В | 64 | 64 | 67 | 64 | 64 | 65 |
| 2 | А | 70 | 67 | 65 | 65 | 65 | 66 | В | 66 | 66 | 65 | 65 | 65 | 65 |
| 3 | А | 71 | 70 | 70 | 69 | 69 | 70 | В | 64 | 67 | 63 | 63 | 67 | 65 |
| 4 | А | 68 | 66 | 65 | 64 | 64 | 65 | В | 67 | 66 | 65 | 65 | 65 | 66 |
| | SLAB "2" | | | | | | | | | | | | | |
| | Cliding | | Swing | | | | Cliding | Swing | | | | | | |
| Edge | direction | 1 | 2 | 3 | 4 | 5 | PTV | direction | 1 | 2 | 3 | 4 | 5 | PTV |
| 1 | А | 72 | 71 | 71 | 70 | 70 | 71 | В | 73 | 73 | 70 | 70 | 70 | 71 |
| 2 | А | 66 | 66 | 65 | 65 | 64 | 65 | В | 66 | 65 | 64 | 64 | 64 | 65 |
| 3 | А | 71 | 71 | 70 | 70 | 70 | 70 | В | 74 | 71 | 70 | 70 | 70 | 71 |
| 4 | А | 70 | 69 | 68 | 68 | 68 | 69 | В | 72 | 71 | 69 | 69 | 69 | 70 |
| SLAB 1:DIRECTION A \rightarrow PTV = 69DIRECTION B \rightarrow PTV = 67PTV = 71SLAB 2:DIRECTION A \rightarrow PTV = 71DIRECTION B \rightarrow PTV = 71PTV = 71 | | | | | | | | '1 '1 | | | | | | |

PTV: New asphalt pavement: ≥ 60 [Centro Sperimentale Interuniversitario di Ricerca Stradale, 2001]







- EXPERIMENTAL RESULTS Macrotexture test
- 2 slabs were tested
- ➤ Tests carried out before (1st test) and after (2nd test) the pendulum test campaign → possible wearing effect due to swings

 $\mathsf{MTD} = 4 \times V / (\pi \times D^2)$

| Slab | MTD | D MTD Decrease | | MTD | | | | |
|------|----------------------|----------------------|-----|------------|--|--|--|--|
| ID | 1 st test | 2 nd test | (%) | Mean value | | | | |
| 1 | 0.57 | 0.50 | 12 | 0.54 | | | | |
| 2 | 0.51 | 0.50 | 2 | 0.51 | | | | |

MTD: New asphalt pavement: ≥ 0.4

[Centro Sperimentale Interuniversitario di Ricerca Stradale, 2001]











| ID | t (mm) | d (mm) | ITS (MPa) | ID | t (mm) | d (mm) | ITS (MPa) |
|--------|--------|--------|-----------|--------|--------|--------|-----------|
| CPu-1 | 45.54 | 123.30 | 1.00 | CPb-1 | 42.98 | 123.30 | 0.69 |
| CPu-2 | 45.31 | 123.30 | 0.98 | CPb-2 | 45.78 | 123.30 | 0.75 |
| CPu-3 | 42.91 | 123.30 | 0.94 | CPb-3 | 45.73 | 123.30 | 0.78 |
| CPu-4 | 43.74 | 123.30 | 1.12 | CPb-4 | 44.78 | 123.30 | 0.72 |
| CPu-5 | 45.62 | 123.30 | 1.10 | CPb-5 | 42.03 | 123.30 | 0.64 |
| CPu-6 | 44.25 | 94.70 | 1.00 | CPb-6 | 46.70 | 94.70 | 0.76 |
| CPu-7 | 48.19 | 94.70 | 1.14 | CPb-7 | 41.33 | 94.70 | 0.85 |
| CPu-8 | 44.92 | 94.70 | 1.15 | CPb-8 | 44.80 | 94.70 | 0.83 |
| ID | t (mm) | d (mm) | ITS (MPa) | ID | t (mm) | d (mm) | ITS (MPa) |
| CMTu-1 | 47.27 | 94.70 | 0.47 | CMTb-1 | 46.41 | 94.70 | 0.7 |
| CMTu-2 | 44.99 | 94.70 | 0.45 | CMTb-2 | 47.54 | 94.70 | 0.57 |
| CMTu-3 | 44.82 | 94.70 | 0.75 | CMTb-3 | 46.99 | 94.70 | 0.56 |
| CMTu-4 | 45.34 | 94.70 | 0.69 | CMTb-4 | 47.55 | 94.70 | 0.62 |
| CMTu-5 | 47.63 | 94.70 | 1.12 | - | - | - | - |
| ID | t (mm) | d (mm) | ITS (MPa) | ID | t (mm) | d (mm) | ITS (Mpa) |
| MMTu-1 | 45.51 | 123.30 | 0.77 | MMTb-1 | 46.93 | 123.30 | 0.98 |
| MMTu-2 | 48.44 | 123.30 | 0.85 | MMTb-2 | 45.89 | 123.30 | 0.91 |
| MMTu-3 | 46.04 | 123.30 | 0.66 | MMTb-3 | 46.72 | 123.30 | 0.35 |
| MMTu-4 | 47.59 | 123.30 | 0.85 | MMTb-4 | 48.90 | 123.30 | 0.63 |
| MMTu-5 | 46.15 | 123.30 | 0.78 | MMTb-5 | 49.11 | 123.30 | 0.67 |
| MMTu-6 | 42.50 | 123.30 | 0.73 | MMTb-6 | 47.23 | 123.30 | 0.41 |

organized by IATT

















Wearing course CMT – CP \rightarrow -34% MMT – CP \rightarrow -26%









Wearing course CMT – CP \rightarrow -34% MMT – CP \rightarrow -26%

Binder course CMT – CP \rightarrow -19% MMT – CP \rightarrow -13%









Wearing course $CMT - CP \rightarrow -34\%$ $MMT - CP \rightarrow -26\%$

Binder course CMT – CP \rightarrow -19% MMT – CP \rightarrow -13%

New asphalt pavement: ITS ≥ 0.6

[Centro Sperimentale Interuniversitario di Ricerca Stradale, 2001]









Wearing course CMT – CP \rightarrow -34% MMT – CP \rightarrow -26%

Binder course CMT – CP \rightarrow -19% MMT – CP \rightarrow -13%

New asphalt pavement: ITS ≥ 0.6

[Centro Sperimentale Interuniversitario di Ricerca Stradale, 2001]

ITS (CMT) < ITS (MMT) \rightarrow Results might be affected by coring activity





CONCLUSIONS

- ➤ The bituminous-cementitious grout shows a fluid consistency → promising for use as monolithic backfilling in mini and micro-trenching systems.
- By quickly setting and hardening, the product might assure fast repairs on road pavements. Such behaviour is confirmed by noticeably mechanical performance after two hours from casting, both in terms of compressive and flexural strengths.
- Surface performance of the material results valuable both in terms of microtexture and macrotexture.
- The adhesion properties at the interface of the existing pavement mini and microtrenching systems were evaluated in terms of ITS on both CMT and MMT samples
 ITS values gathered testing CMTu and MMTu samples comply with minimum requirements of Italian technical Standard for asphalt concrete pavements.

In conclusion, from this study on Flowmix Fast as backfilling material for mini-microtrenching applications, promising surface and interface adhesion properties were observed. Future development of the research will focus on the adhesion properties of the grout, testing the interface under repeated dynamic loads







dal 1980 l'edilizia in buone mani







Thank you for your attention

organized by IATT